باسمه تعالی ۱۷/۳/۱۷

تحلیل و طراحی ستونها

ترکیب ستون: عضو فشاری + خمش

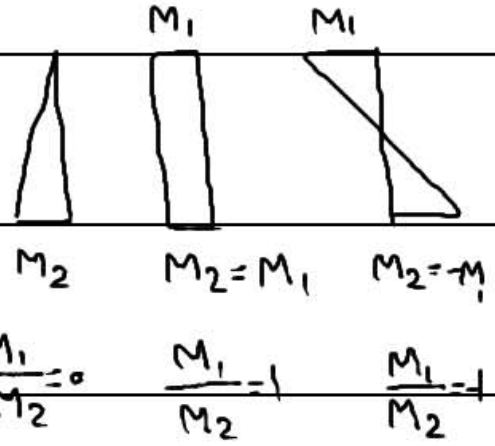
ستون ← کوتاه
← بلند، لاغر

$\lambda = \frac{k l_u}{r} \leq 34 - 12\left(\frac{M_1}{M_2}\right)$ بدون انتقال جانبی

r ← $\sqrt{\frac{I}{A}}$ ← $.3h$ / $.25d$

$|M_2| > |M_1|$

$\lambda \leq 22$



خاموت (دورگیری): جلوگیری از کمانش میلگردهای طولی

خاموت برشی

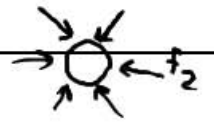
ثابت نگه داشتن فاصله میلگردهای طولی هنگام اجرا

تنگ دار
مارپیچ

دورگیری بتن هسته مرکزی ← افزایش مقاومت بتن

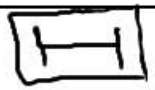
$f_1 = f_c + 4.1 f_2$

← افزایش شکل پذیری



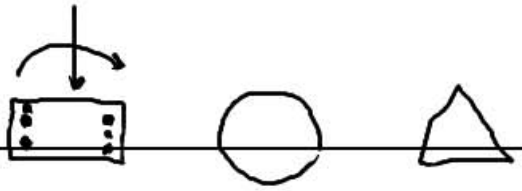
از نظر مسلح کردن ← میلگرد طولی

پروفیل فولادی ← محیط / محاط



مسائل ← تحلیل: ظرفیت ستون با ابعاد و آرماتور مشخص پیدا کنیم

↓ طراحی

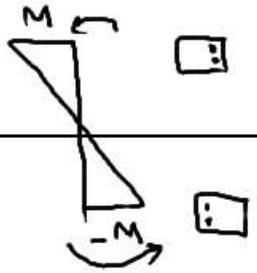
تحلیل ستون:  بنروی محوری + خمش

دلایل ضعف آرماتورگذاری کردن ستونها:

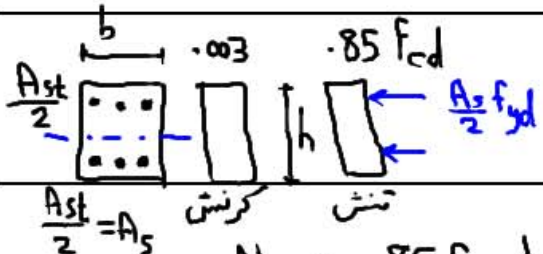
① لنگر بخاطر بادیا زلزله ±

② انحنای مضاعف ستونها

③ سادگی اجرا



ستون: بنروی محوری + خمش



① ستون تحت اثر بنروی محوری

$$N_{r_{max}} = .85 f_{cd} bh + (f_{yd} - .85 f_{cd}) A_{st}$$

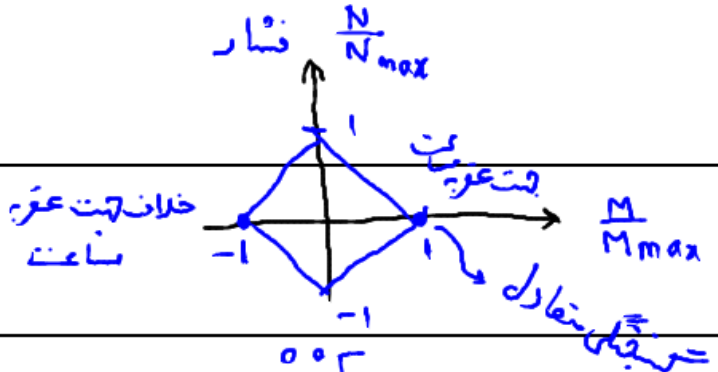
خمش + بنروی محوری:

$$\frac{N}{A} + \frac{My}{I} = f_{cu} \longrightarrow \frac{N}{f_{cu} A} + \frac{M}{\frac{I f_{cu}}{y}} = 1$$

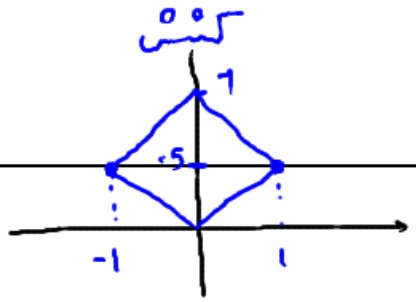
رابط اندرکنش لنگر-بنروی محوری

$$\frac{N}{N_{max}} + \frac{M}{M_{max}} = 1$$

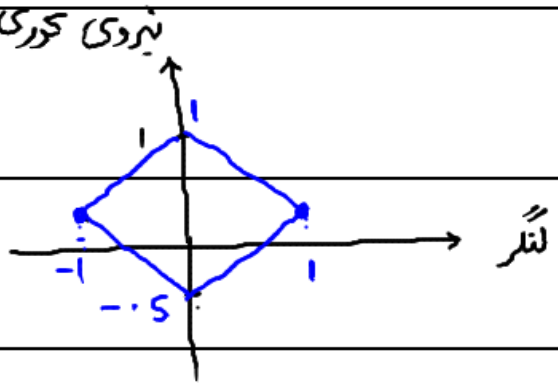
$|f_{cu}| = |f_{tu}|$



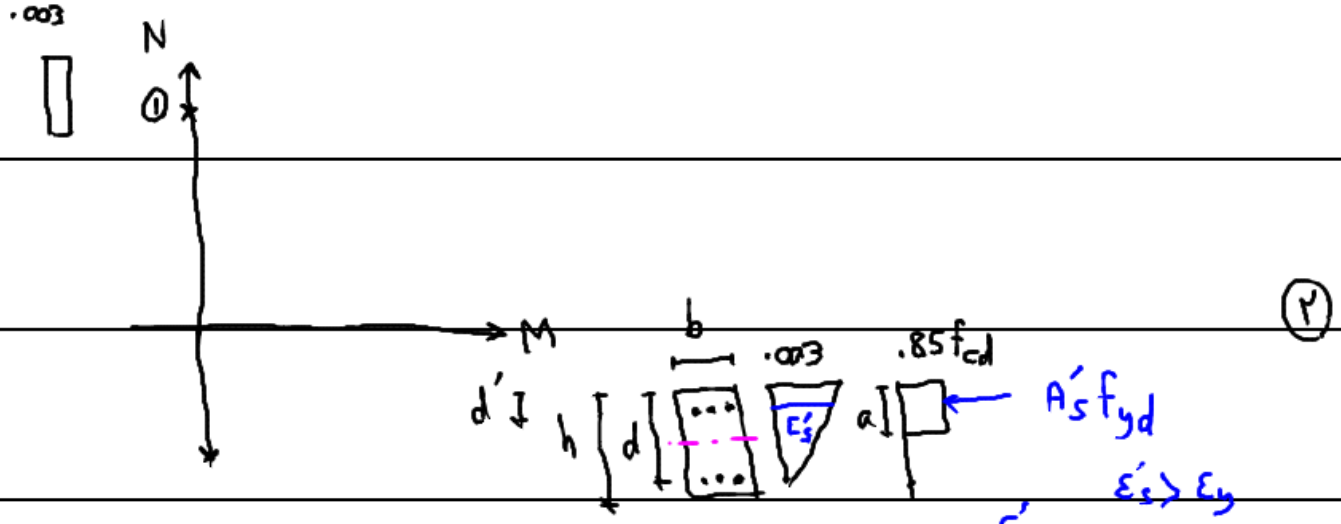
$f_{tu} = 0$



$|f_{tu}| = .5|f_{cu}|$



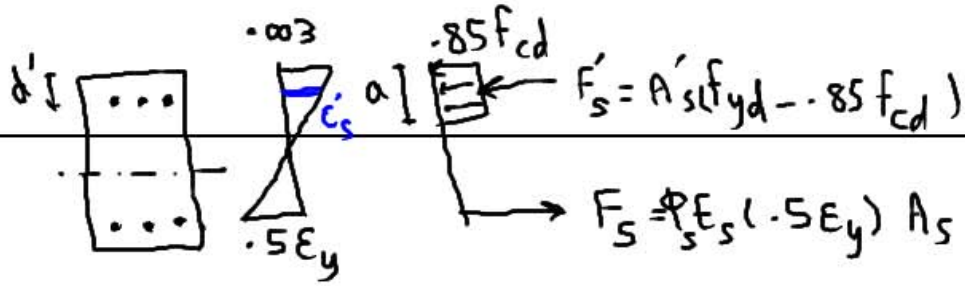
$N_{r_{max}} = A_{st} f_{yd}$ نسبی



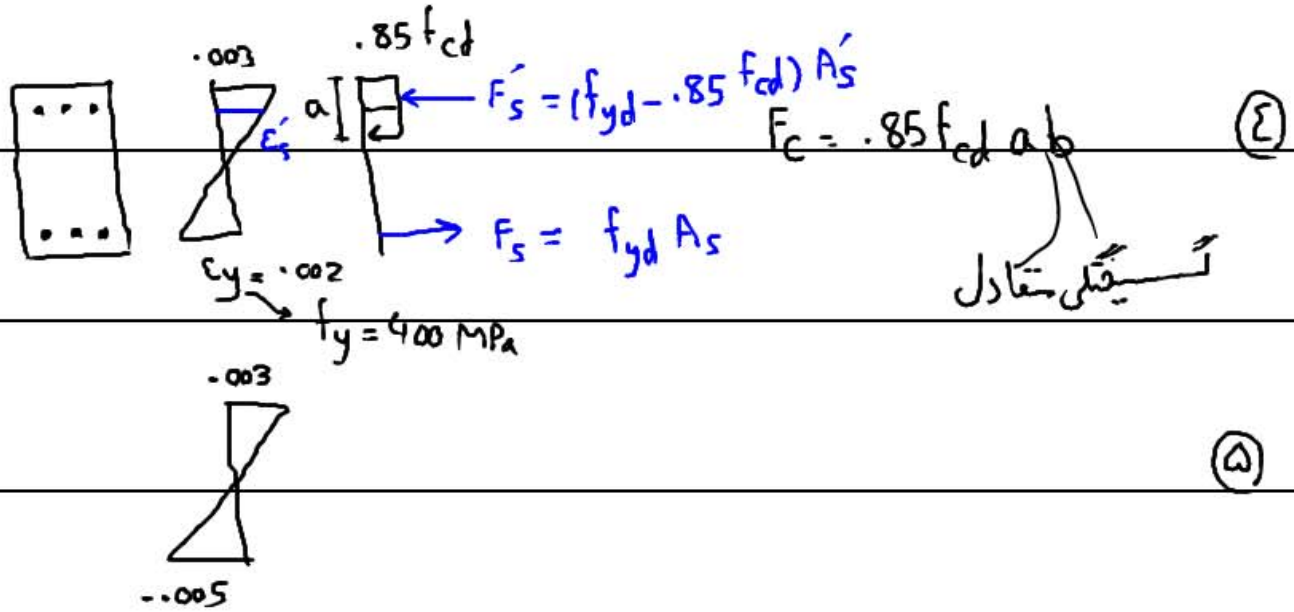
(۲)

$$N_r = \overbrace{.85 f_{cd}\, ab}^{F_c} + \overbrace{(f_{yd} - .85 f_{cd})\, A'_s}^{F'_s}$$

$$M_r = F_c\left(\frac{h}{2} - \frac{a}{2}\right) + F'_s\left(\frac{h}{2} - d'\right)$$



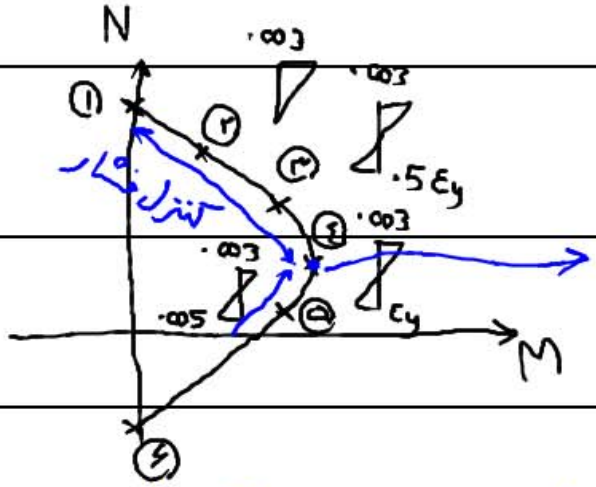
(۳)

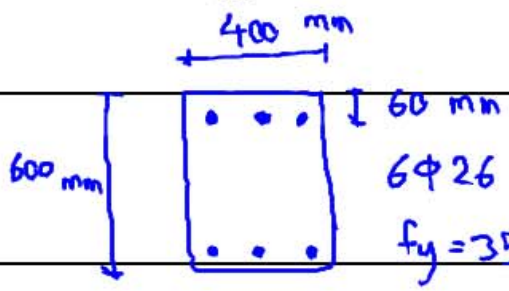


(۴)

(۵)

$N_{rt} = A_{st} f_{yd}$ کشش خالص (۶)





$A'_s = A_s = 3\Phi 26 = 1593\ mm^2$

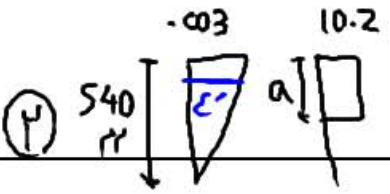
تکیل ستون.

$f_c = 20$ MPa ⟶ $f_{cd} = 12$ MPa

$f_{yd} = .85 \times 350 = 298$ MPa

① $N_{r_{max}} = .85 \times 12 \,(400 \times 600) + (298 - .85 \times 12)(2 \times 1593)$

$N_{r_{max}} = 3365$ KN.

② 

$a = .85 \times 540 = 459$ mm

$.65 \leq \beta_1 = 1.09 - .008 f_c \leq .85$

$\frac{\varepsilon'_s}{.003} = \frac{480}{540} \rightarrow \varepsilon'_s = .0027 > \varepsilon'_y = \frac{350}{200000} = .0018$

$F_c = 10.2 \times 400 \times 459 = 1873$ KN

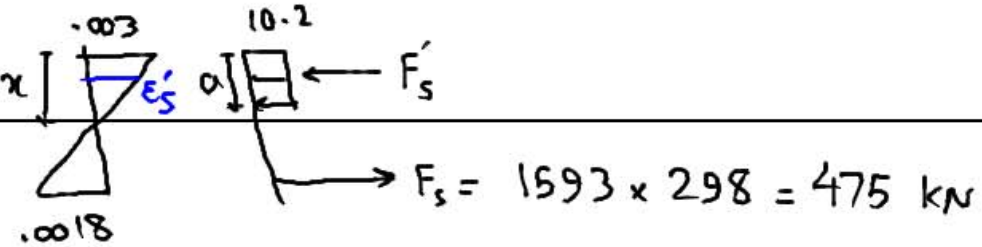
$F'_s = 1593\,(298 - 10.2) = 458$ KN

$F_s = 0$

$N_r = 1873 + 458 = 2331$ KN

$M_r = 1873\left(\frac{600}{2} - \frac{459}{2}\right) + 458\left(\frac{600}{2} - 60\right) = 242$ KN·m

2331	KN
242	KN·m

④ 

$F_s = 1593 \times 298 = 475$ KN

$\frac{.003}{x} = \frac{.0018}{540 - x} = \frac{.0048}{540} \rightarrow x = 338$ mm

$a = \beta_1 x = .85 \times 338 = 287 \text{ mm}$ $\quad F_c = 10.2 \times 400 \times 287 = 1171 \text{ KN}$

$$\frac{\varepsilon_s'}{.003} = \frac{278}{338} \rightarrow \varepsilon_s' = .0025 > .0018$$

$$F_s' = 1593\,(298 - 10.2) = 458 \text{ KN}$$

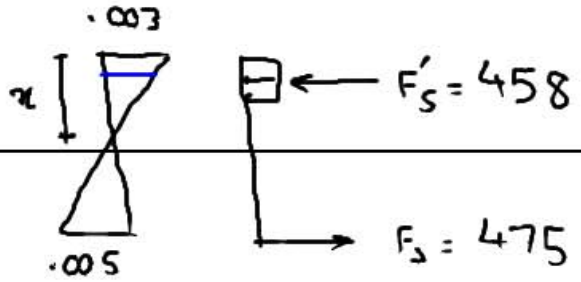
$$N_r = 1171 + 458 - 475 = 1154 \text{ KN}$$

$$M_r = 1171\left(300 - \frac{287}{2}\right) + 458\,(300 - 60) + 475\,(300 - 60)$$

$$M_r = 407 \text{ KN.m}$$

1154 KN
407 KN.m

⑤



$$\frac{.003}{x} = \frac{.005}{540 - x} = \frac{.008}{540} \qquad x = 203 \text{ mm}$$

$a = .85 \times 203 = 172 \text{ mm}$

$$F_c = 10.2 \times 400 \times 172 = 702 \text{ KN}$$

$$N_r = 458 + 702 - 475 = 685 \text{ KN}$$

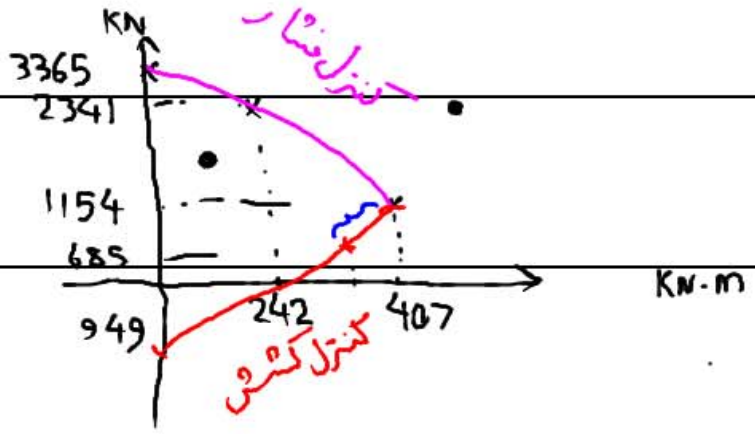
$$M_r = 702\left(300 - \frac{172}{2}\right) + 458\,(300 - 60) + 475\,(300 - 60)$$

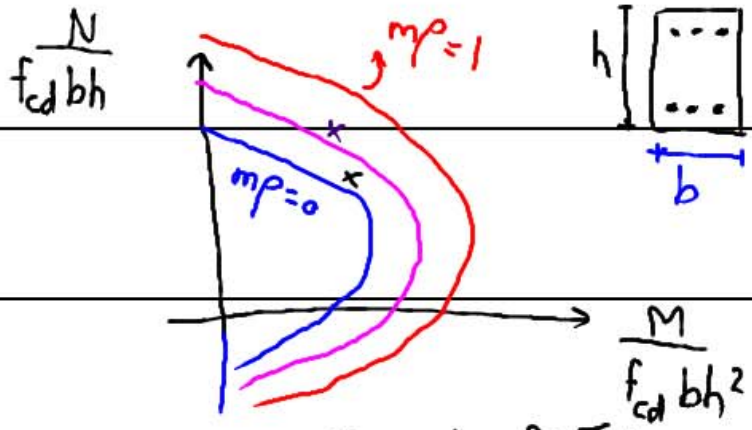
$$M_r = 374 \text{ KN.m}$$

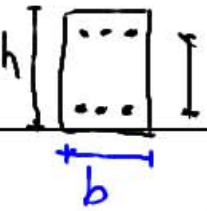
685 KN

374 KN.m

۶) $N_r = (1593 \times 2) \times 298 = 949$ KN







$\gamma = .8$

$m\rho = \frac{f_{yd}}{.85 f_{cd}} \times \frac{A_{st}}{A_g}$

طراحی: استفاده از نمودارهای اندرکنش بی بعد شده